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## Girder System Employing Bent Steel Plating

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**Azizinamini**

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(54) **GIRDER SYSTEM EMPLOYING BENT STEEL PLATING**

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(73) Assignee: **Board of Regents of University of Nebraska**, Lincoln, NE (US)

(\*) Notice: Subject to any disclaimer, the term of this patent is extended or adjusted under 35 U.S.C. 154(b) by 208 days.

(21) Appl. No.: **11/405,107**

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**Related U.S. Application Data**

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(51) **Int. Cl.**  
**E01D 2/04** (2006.01)

(52) **U.S. Cl.** ..... **14/74.5**; 29/897.35

(58) **Field of Classification Search** ..... 14/74.5,  
14/78; 29/897.35

See application file for complete search history.

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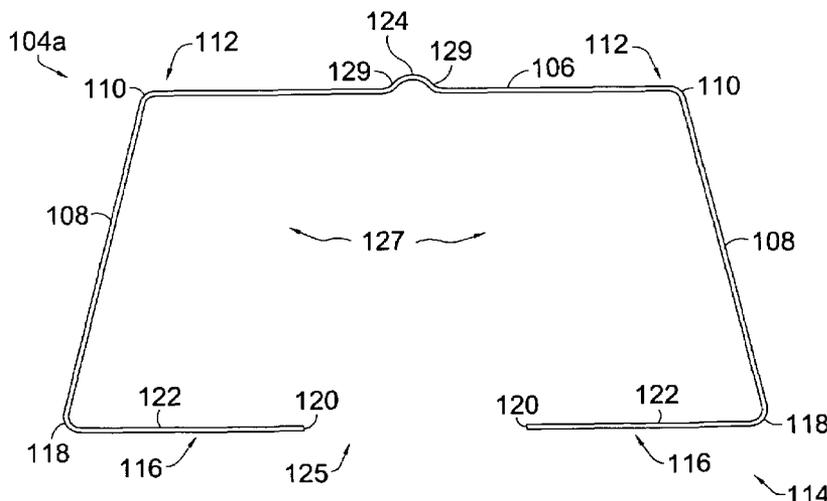
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(57) **ABSTRACT**

A structural support system employs a modified inverted box girder design for improved performance and ease of fabrication. Each girder of a series of girders is formed with an upper flange section, a set of web sections extending from opposite lateral sides of the upper flange section, and a base section below the web sections. The base section includes individual flanged footings that project generally towards one another. The particular cross-sectional configuration of each girder may be formed by bending a continuous steel plate along at least a first set of preselected parallel lines to establish a set of upper continuous transitional bends with the upper flange section therebetween, as well as a remainder portion forming at least the set of web sections. A deck is coupled with the series of girders to form the structural support system.

**15 Claims, 3 Drawing Sheets**



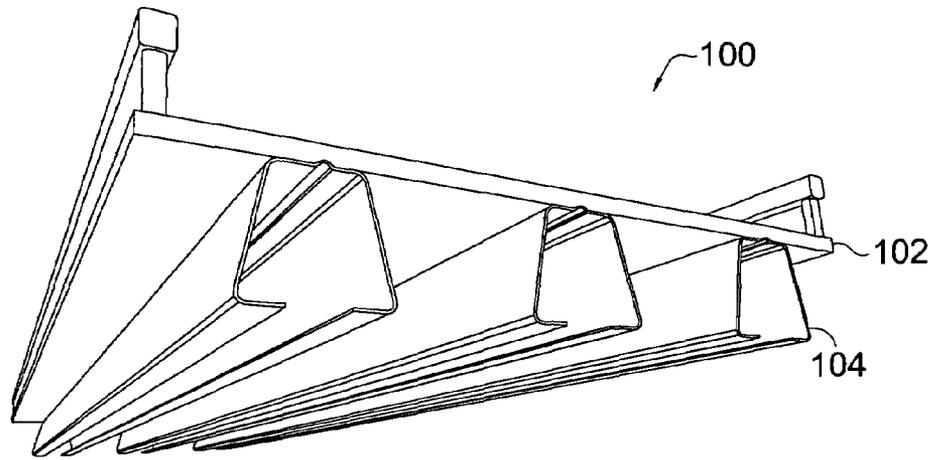


FIG. 1.

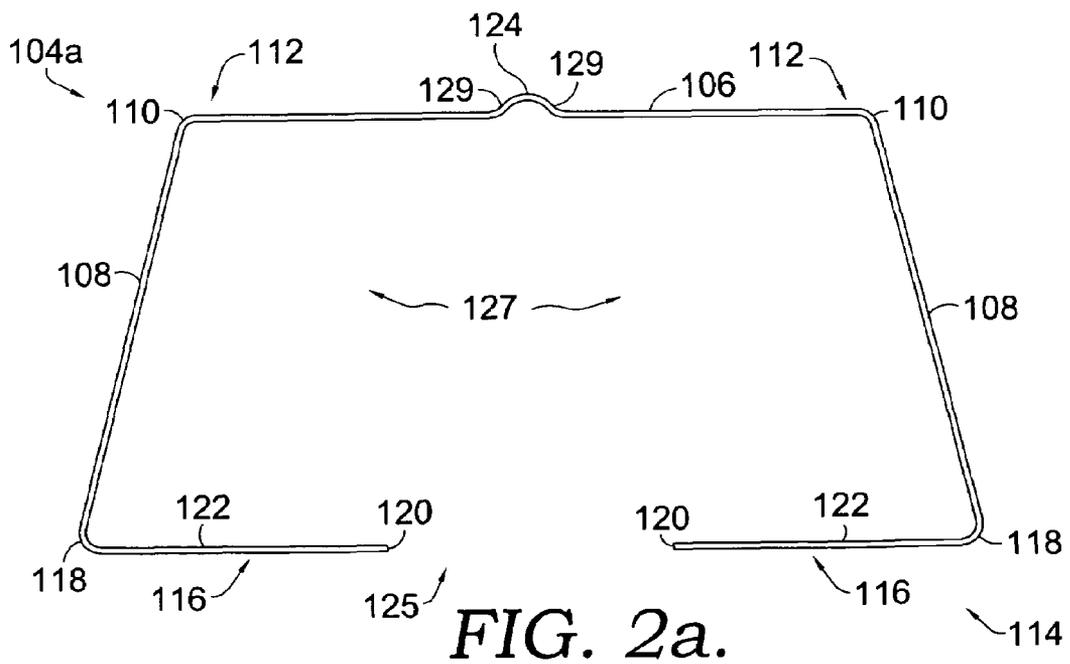


FIG. 2a.

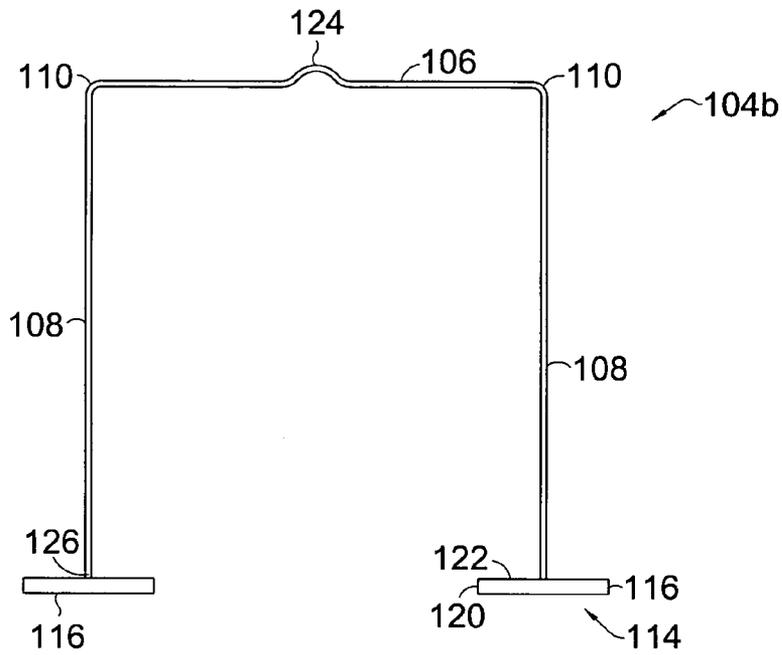


FIG. 2b.

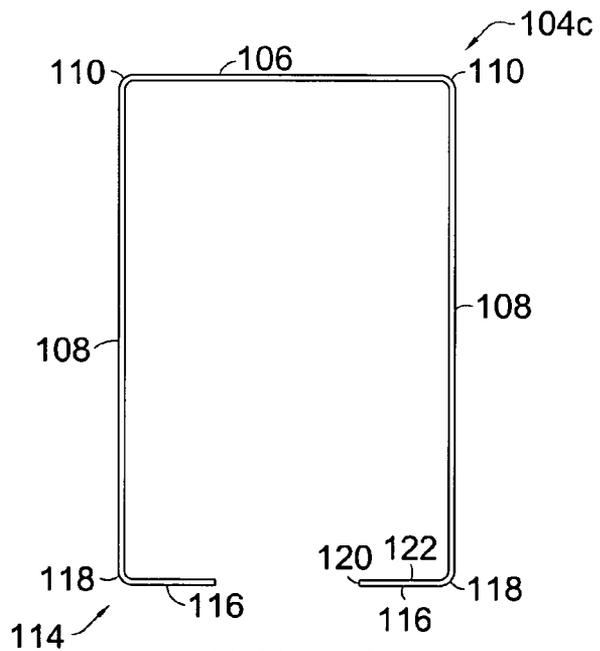
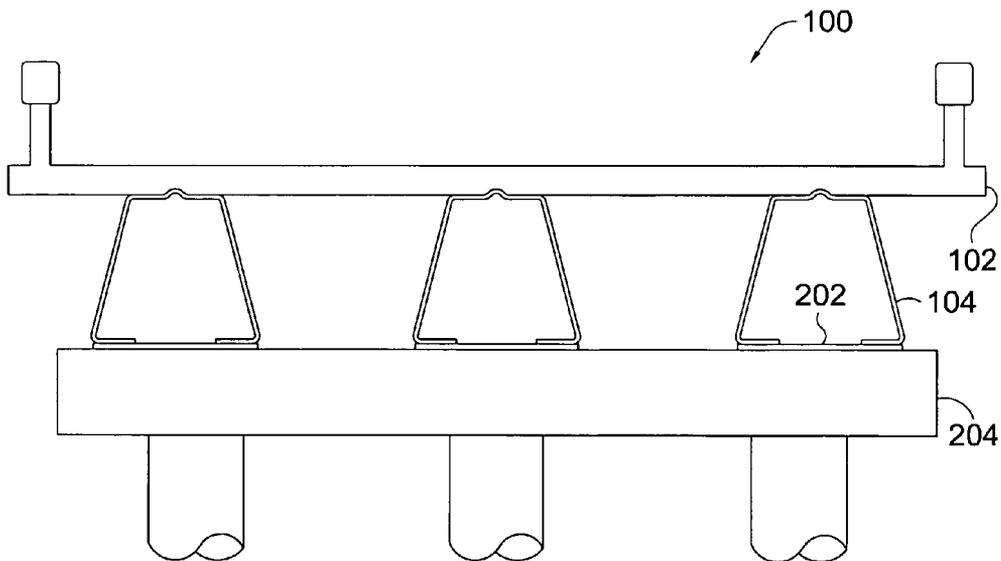
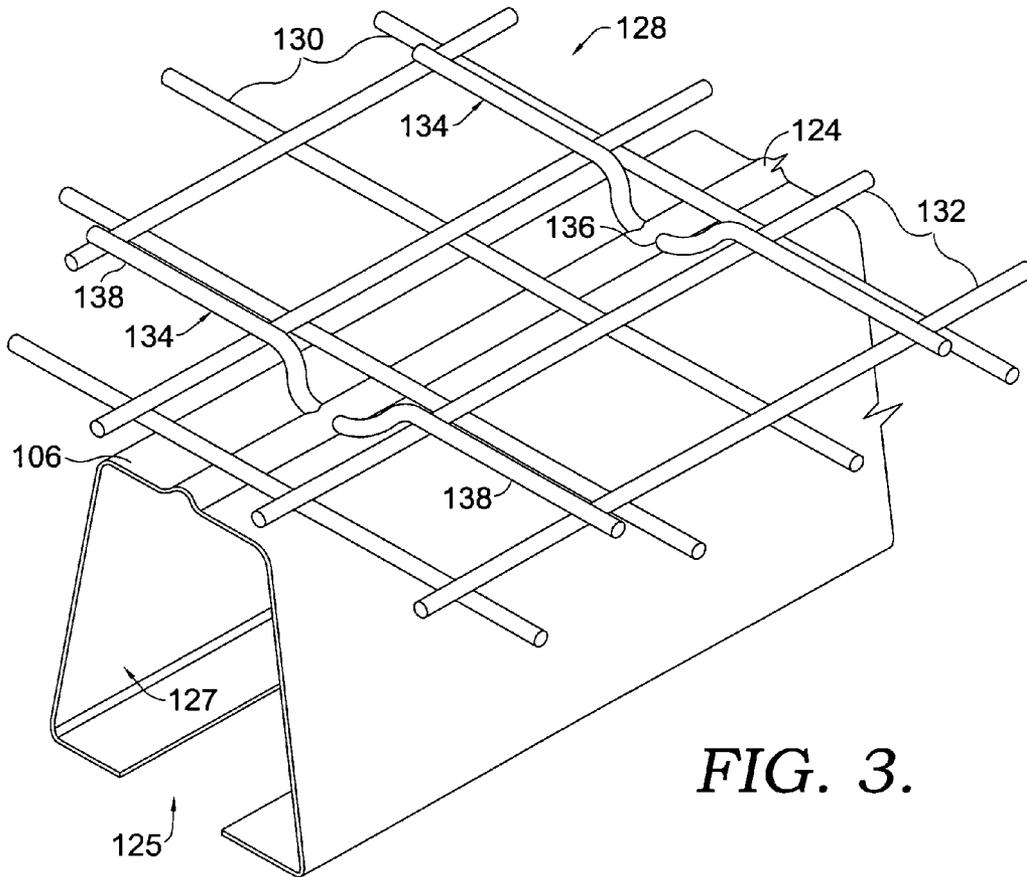


FIG. 2c.



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**GIRDER SYSTEM EMPLOYING BENT STEEL  
PLATING****CROSS-REFERENCE TO RELATED  
APPLICATIONS**

This application claims priority to commonly owned U.S. provisional application Ser. No. 60/671,736, filed Apr. 15, 2005, incorporated by reference in its entirety.

**STATEMENT REGARDING FEDERALLY  
SPONSORED RESEARCH OR DEVELOPMENT**

Not applicable.

**BACKGROUND OF THE INVENTION**

Structural support systems, such as those used in buildings or bridges, often incorporate steel girders to form a load-carrying element. For instance, in the case of bridges, the generally horizontally extending girders provide support to an overlying superstructure (e.g., a bridge deck) and transfer loads to support columns or other structures anchored to the ground. Girders have a variety of cross-sectional configurations, such as I-beam and box-shaped, each providing specific advantages depending on the particular design parameters for a load carrying system. In certain situations, for example, it is desirable to maximize the strength-to-weight ratio for a girder design, while other situations dictate the incorporation of girders that are, above other considerations, easy to fabricate with low maintenance over time.

Despite advances in design for load carrying systems, conventional fabrication methods for steel girders are still generally labor intensive, and result in a finished product that is relatively expensive. With both I-beam and box girder configurations, for example, there is a large amount of welding that must be done to secure the main structural components of the girder together. Furthermore, because of the extent of welding that is necessary, there is frequently a long delay introduced from the time a particular girder design is chosen to the time a load carrying system is constructed on-site. Bridges employing girders having certain cross-sectional configurations also require cross frames to be coupled between adjacent girders in order to maintain sufficient structural stiffness of the load carrying system. These cross frames add to the expense, maintenance cost and labor of bridge installation.

Overall, there is increased desire for load carrying systems that can be more rapidly deployed at a relatively low cost. In certain applications, for example, pre-assembly of significant components of a load carrying system, such as a bridge deck and supporting girders, would provide for faster installation of a bridge system. It would also be advantageous to have a fabrication method for steel girders in which an individual girder pattern could be easily duplicated such that a series of matching girders may be formed.

**SUMMARY OF THE INVENTION**

A structural support system is provided utilizing bent steel plate girders. The girders employ a modified inverted box cross-sectional configuration to provide a longitudinal load-carrying member with good structural stability and relative simplicity in fabrication. In one aspect, the girder is formed with an upper flange section, a set of web sections extending from opposite lateral sides of the upper flange section, and a base section below the web sections. The upper flange section

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may be formed with a longitudinal ridge to increase the buckling capacity of the girder, as well as provide a feature for physically coupling the girder with an overlying superstructure, such as a bridge deck, to form the structural support system. A set of upper transitional bends form the interface between the upper flange section and the web sections. The base section includes individual flanged footings that project generally towards one another.

In another aspect, the aforementioned girder configuration may be embodied in various cross-sectional forms. For instance, the set of web sections may be parallel with one another, or may be arranged in a divergent relationship moving away from the upper flange section. As another variation optimally for longer span girders, the flanged footings, in addition to projecting towards one another, also project in opposed directions from a lower edge of each of the web sections, such that each web section is straddled by one of the flanged footings.

The cross-sectional configuration of girder, in another aspect, is formed by bending a continuous steel plate along at least a first set of preselected parallel lines to establish the set of upper continuous transitional bends and the upper flange section therebetween, as well as a remainder portion forming at least the set of web sections. Optionally, the remainder portion of the steel plate may be bent along a second set of preselected parallel lines to establish a set of lower continuous transitional bends that defines individual flanged footings of the base section extending from the web sections. Alternatively, flanged footings may be welded or otherwise secured onto a lower edge of each of the web sections. As a further option, an upward bend generally parallel with the first set of preselected parallel lines may be formed along a centerline of the upper flange section to establish a longitudinal ridge for physically coupling the girder with an overlying superstructure. This arrangement also allows a concrete slab deck encasing a grid of reinforcing members to optionally be formed simultaneously with the coupling of a series of girders to the superstructure deck by extending the reinforcing members through the longitudinal ridge of each girder upper flange section.

As described, the structural support system possessing a modified inverted box cross-sectional configuration, and method of assembly therefore, provide a rapidly deployable load-carrying system with a reduced number of fabrication steps as compared to conventional methods. Particularly in the case of the girder formed entirely by bending processes, an individual girder pattern may be selected and quickly duplicated to generate a series of matching girders for a given bridge span or other structural support system design. Structural support systems of the present invention may be formed by joining a series of girders with an overlying deck at an installation site, or may be deployed to an installation site as a preassembled deck/girder unit.

Additional advantages and features of the invention will be set forth in part in a description which follows, and in part will become apparent to those skilled in the art upon examination of the following, or may be learned by practice of the invention.

**BRIEF DESCRIPTION OF THE SEVERAL  
VIEWS OF THE DRAWINGS**

In the accompanying drawings, which form a part of the specification and are to be read in conjunction therewith and in which like reference numerals are employed to indicate like parts in the various views:

FIG. 1 is a perspective view of a structural support system incorporating a series of longitudinally oriented girders coupled to a superstructure;

FIGS. 2a, 2b and 2c show cross-sectional profiles taken transversely through various embodiments of the longitudinally oriented girders;

FIG. 3 is a fragmentary perspective view of a grid of reinforcing members of a superstructure coupled with a feature of an upper flange section of one of the girders; and

FIG. 4 is a front elevational view of the structural support system installed on a column support.

#### DETAILED DESCRIPTION OF THE INVENTION

Turning now to the drawings, and initially to FIG. 1, a structural support system 100 is depicted for providing a load-carrying element. The system 100 is particularly well suited for supporting loads upon a deck 102 supported by a series of underlying girders 104 that extend generally in a horizontal longitudinal orientation, such as in the form of a bridge span or building floor span. It should be understood, however, that the orientation of the deck 102 and underlying girders 104 may vary from the horizontal, such as in the case of a bridge span having a convex or concave longitudinal profile with respect to the horizontal plane, or ascending or descending in elevation longitudinally over the span. Additionally, descriptions of deck 102 or girder 104 components or features as being “upper” or “lower”, and orientations or interrelations of such components or features described as “upwardly”, “downwardly”, “underlying” or “overlying”, are with reference to the deck 102 and girder 104 extending in a horizontal longitudinal arrangement. Those of skill in the art will appreciate how such terminology is adjusted when the system 100 and components thereof are utilized in other spatial arrangements, such as in a vertically upturned arrangement, as will be discussed hererin.

With continued reference to FIG. 1, and additional reference to FIGS. 2a-2c, various embodiments of the girder 104 (also labeled as girder 104a, 104b, or 104c in correspondence with FIGS. 2a-2c) are illustrated in a cross-sectional profile. Each embodiment of the girder 104 includes an upper flange section 106, a set of web sections 108 extending downwardly from a set of upper continuous transitional bends 110 formed on opposite lateral sides 112 of the upper flange section 106, and a base section 114 including individual flanged footings 116 formed below the set of web sections 108. The girders 104 illustrated in FIGS. 2a-2c may be fabricated either partially or wholly by a bending process of a continuous steel plate, as will be explained in further detail herein.

The embodiment of the girder 104a illustrated in FIG. 2a, and shown for exemplary purposes coupled to the bridge deck 102 of FIG. 1, also includes a set of lower continuous transitional bends 118 separating each web section 108 from one of the flanged footings 116. The flanged footings 116 project generally laterally towards one another, each extending to a terminal edge 120. The flanged footings 116 may also have a flat profile or a slightly downwardly sloped profile moving towards the terminal edge 120 to allow any moisture that collects onto a top surface 122 of the footings 116 to drain off of the terminal edge 120 when the girder 104 and deck 102 are installed in the horizontal longitudinal arrangement. In the particular embodiment of FIG. 2a, the web sections 108 of the girder 104a are aligned in a divergent relationship with one another moving away from the upper flange section 106. However, the web sections 108 may also be parallel with one another, as in the embodiments of the girder 102b and 102c illustrated in FIGS. 2b and 2c, respectively. Those of skill in

the art will appreciate that the relative web section 108 orientations are a matter of design choice based on the desired load-carrying capacity of the girder, and other engineering design factors. The girder 104a also has a longitudinally extending ridge 124 positioned along a longitudinal centerline of the upper flange section 106. The ridge 124 is raised upwardly above the plane of the upper flange section 106, and serves to provide increased buckling capacity from the girder 104a while also providing a feature for physically coupling the deck 102 with the girder 104a, as will be explained herein with reference to FIG. 3.

The embodiments of the girders 104b and 104c illustrated in FIGS. 2b and 2c, respectively, share some of the same features with the girder 104a of FIG. 2a. For instance, girder 104b of FIG. 2b has the longitudinally extending ridge 124 formed on the upper flange section 106, while the girder 104c of FIG. 2c includes the set of lower continuous transitional bends 118 between the web sections 108 and the flanged footings 116. The girder 104c also possesses flanged footings 116 projecting in the same orientation as with the girder 104a of FIG. 2a. On the other hand, the girder 104b of FIG. 2b differs from both of the girders 104a and 104c of FIGS. 2a and 2c, respectively, by having each flanged footing 116 arranged to straddle a lower edge 126 of one of the web sections 108. In each embodiment, it is preferred to have some lateral distance between the flanged footings 116 so as to create a continuous channel opening 125 therebetween. This not only provides the advantage of a longitudinal gap where moisture can drain off of the top surface 122 of the footings 116, but if sufficiently large, the opening 125 allows for persons to inspect the inner surface region 127 of the girder 104 for cracks, corrosion, or other degradation that can occur over time. With a traditional box girder, for example, such an inspection may be difficult, if not impossible, due to a lack of access to the girder inner surface region.

As referenced above, each girder profile 104 illustrated in FIGS. 2a-2c may be fabricated at least partially by a bending process of a continuous steel plate. In such a case, the longitudinal dimension of the girder is limited by the length of a steel plate attainable, and may further be limited by size of plate bending equipment that is utilized. In one exemplary arrangement, girder longitudinal spans of up to 105 feet are contemplated based on a steel plate thickness of one-half of an inch. However, other girder spans and steel plate thicknesses are contemplated by the present invention, and any specific dimensional values recited herein should not be construed as limiting the scope of the present invention. A steel plate thickness of three-eighths of an inch for a girder longitudinal span of up to 50 feet, for instance, is another potential configuration. Steel plates chosen to form the girder 104 include those made of high quality carbon steel, with one particular material choice being A1010 martensitic steel due to its resistance to corrosion.

In the bending process, a first set of parallel lines are selected to correspond to the locations where a steel plate is bent to form the upper continuous transitional bends 110, with the upper flange section 106 established between the bends 110 and presenting a flange section width selected according to the location of the bends 110. In the case of the girders 104a and 104c of FIGS. 2a and 2c, the process also involves the selection of a second set of parallel lines corresponding to the locations where the remaining portion of the steel plate is bent to form the lower continuous transitional bends 118. This step establishes the “height” or dimension of each web section 108 between the upper and lower continuous transitional bends 110, 118, as well as the dimension of each flange footing 116 between the lower continuous tran-

sitional bend **118** and the terminal edge **120** of each footing **116**. Optionally, the longitudinally extending ridge **124** is formed by bending the upper flange section **106** along a third set of parallel lines **129**. The aforementioned bending process involving the formation of upper and lower transitional bends **110**, **118** may be accomplished by plate bending equipment under the control of an automated system. For instance, equipment incorporating computer numeric control (CNC) may be used to select a particular girder profile and then implement a bending process to form the particularly dimensioned girder **104** repeatedly, if desired.

An alternative fabrication process takes place for the girder **104b** of FIG. 2*b*. After the upper continuous transitional bends **110** are formed, the remaining portion of the steel plate serves as the web sections **108**, each terminating at the lower edge **126**. A flange footing **116** is then welded onto the lower edge **126** of each web section **108** such that the lower edge **116** straddles the footing **116**. The footing **116** may have an increased thickness over the remainder of the girder **104b**, depending on strength and stability requirements.

One particular arrangement for coupling the girders **104a** and **104b** with the deck **102** involves the use of reinforcing members **128** encased with the concrete deck **102**, shown in FIG. 3 without the deck **102** in place. The reinforcing members **128** may be steel reinforcing bars commonly used in road and bridge construction. The concrete deck **102** includes a grid of reinforcing members **128** extending generally in the horizontal plane in alternating layers characterized by transversely extending members **130** in one layer and longitudinal members **132** in either an overlying or underlying layer with respect to the transverse members **130**. Particular coupling members **134** of the reinforcing members **128** are each shaped to extend through an aperture **136** formed generally transversely through the longitudinally extending ridge **124**, with opposed sides **138** bending upwardly and over the longitudinal members **132**. A plurality of apertures **136** are spaced along the ridge **124** in the longitudinal direction. Once the coupling members **134** are installed through each of the apertures **136** of the longitudinally extending ridge **124**, concrete may be cast around the reinforcing members **128** to encase the member **128** and form the deck **102** overlying the upper flange section **106** and the longitudinally extending ridge **124**. This configuration provides the advantage of establishing an interlocking composite action between the deck **102** and the girders **104** to more evenly distribute stresses throughout the system **100** while preventing lateral movement of the deck **102** relative to the girders **104**.

As an alternative coupling arrangement between the deck **102** and the girder **104**, if the longitudinally extending ridge **124** is not incorporated into the upper flange section **106**, other means known to those of skill in the art may be employed to physically couple the girders **104** with the deck **102**. For instance, a threaded bolt (not shown) may be screwed downwardly through the concrete deck **102** and fastened through an aperture (not shown) in the upper flange section **106** to secure the deck **102** to the girders **104** and prevent lateral movement of the deck **102** relative to the girders **104**.

With reference to FIG. 4, the system **100** including bridge deck **102** and series of girders **104** is shown within an exemplary completed bridge installation **200**. The girders **104** are mounted upon a series of bearing plates **202** that overlie a column support **204**. Specifically, the flanged footings **116** of each girder **104** rest upon the bearing plates **202**. It should be

understood that the bridge installation **200** is merely illustrative of one exemplary arrangement incorporating the system **100** of the present invention.

As mentioned above, it is contemplated that the system **100** may be oriented such that the deck **102** extends in a vertical plane and the girders **104** extend in either a horizontal longitudinal alignment, one above the other, or in a vertical longitudinal alignment. In other words, the system **100** may be turned upwardly and secured to a stable structure to act as a retaining wall, in one example, for earthen material.

As can be understood, the structural support system **100** of the present invention incorporating the particular girder **104** design results in a load carrying structure that may be easily assembled and rapidly deployed in a bridge installation. Furthermore, the present invention provides a low maintenance girder **104** that can be easily inspected for structural integrity when the need arises.

Furthermore, since certain changes may be made in the above invention without departing from the scope hereof, it is intended that all matter contained in the above description or shown in the accompanying drawing be interpreted as illustrative and not in a limiting sense. It is also to be understood that the following claims are to cover certain generic and specific features described herein.

What is claimed is:

1. A load-carrying girder presenting a longitudinal span for supporting an overlying superstructure, comprising:
  - an upper flange section defining a bearing surface, the upper flange section having a longitudinally extending ridge;
  - a set of upper continuous transitional bends formed on opposed lateral sides of the upper flange;
  - a set of web sections spaced apart from one another, each web section extending downwardly from one of the transitional bends; and
  - a base section formed below and extending from the set of web sections, the base section including individual flanged footings projecting generally laterally towards one another;
 wherein the ridge of the upper flange is formed with a set of laterally extending, spaced apart apertures formed there-through, the apertures being configured to receive a reinforcing member.
2. The girder of claim 1, wherein the base section further includes:
  - a set of lower continuous transitional bends each formed at a bottom portion of one of the web sections of the set of web sections, the flanged footings extending from the set of lower continuous transitional bends.
3. The girder of claim 2, formed by a process including the steps of:
  - bending a continuous steel plate along a first set of preselected parallel lines to establish the set of upper continuous transitional bends and the upper flange section between the set of upper continuous transitional bends; and
  - bending the continuous steel plate along a second set of preselected parallel lines to establish the set of lower continuous transitional bends, the set of web sections between the set of upper continuous transitional bends and the set of lower continuous transitional bends, and the flanged footings.
4. The girder of claim 1, wherein the set of web sections are arranged in a divergent relationship with one another moving away from the upper flange section.

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5. The girder of claim 1, wherein the flanged footings of the base section are configured such that a continuous channel opening is formed therebetween.

6. A structural support system comprising:

one or more longitudinally oriented girders, each girder having an upper flange section defining a bearing surface, a set of web sections extending downwardly from transitional bends formed on opposed lateral sides of the upper flange section, and a base section formed below the set of web sections, the base section including individual flanged footings projecting generally laterally towards one another; and

a deck coupled to the one or more girders through the upper flange section, the deck having a bottom surface overlying the bearing surface of the series of girders;

wherein the upper flange section of each girder is formed with a longitudinally extending ridge having a set of spaced apart apertures through which the deck couples with the series of girders.

7. The support system of claim 6, wherein the set of web sections of each girder are arranged in a divergent relationship with one another moving away from the upper flange section.

8. The support system of claim 6, wherein the flanged footings of the base section of each girder are configured such that a continuous channel opening is formed therebetween.

9. The support system of claim 6, formed by a process including the steps of:

a) fabricating each girder of the one or more girders, the step of fabrication including bending a continuous steel plate along a plurality of preselected longitudinal lines to form at least the upper flange section and the set of web sections of a respective girder;

b) fabricating the deck as a concrete slab encasing a grid of reinforcing members; and

c) coupling the deck to the series of girders; wherein steps (b) and (c) are performed either simultaneously or separately.

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10. The support system of claim 9, wherein process step (a) further includes forming the base section through the fabrication step of bending the continuous steel plate along the plurality of preselected longitudinal lines.

11. The support system of claim 9, wherein process step (c) includes extending at least some of the reinforcing members through a raised feature of the upper flange section of each girder.

12. The support system of claim 11, wherein the raised feature of the upper flange section comprises a longitudinally extending ridge formed with a set of spaced apart apertures.

13. A method for fabricating a load-carrying element of a structural support system, comprising:

bending a series of continuous steel plates along a plurality of preselected longitudinal lines to form a series of load-carrying girders, each girder having an upper flange section defining a bearing surface, a set of web sections extending downwardly from transitional bends formed on opposed lateral sides of the upper flange section, and a base section formed below the set of web sections, the base section including individual flanged footings;

forming a concrete slab deck encasing a grid of reinforcing members; and

coupling the deck to each girder of the series of girders such that the series of girders extend in a parallel arrangement with respect to one another; and

wherein coupling the deck to each girder includes extending at least some of the reinforcing members through a raised feature of the upper flange section of each girder.

14. The method of claim 13, wherein the flanged footings of the base section project generally laterally towards one another.

15. The method of claim 13, wherein the raised feature of the upper flange section comprises a longitudinally extending ridge formed with a set of spaced apart apertures.

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